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# **European Technical Assessment**

ETA 14/0294 of 21/08/2014

Technical Assessment Body issuing the ETA and designated according to Article 29 of the Regulation (EU) No 305/2011

Technical and Test Institute for Construction Prague

Trade name of the construction product

edilon)(sedra Dex<sup>®</sup>-EA 2K, edilon)(sedra Dex<sup>®</sup>-EA 2KC edilon)(sedra Dex<sup>®</sup>-EA 2KH steel bonded anchor

Product family to which the construction product belongs

Product area code: 33
Bonded injection type anchor for use in cracked and non-cracked concrete

Manufacturer

edilon)(sedra bv Nijverheidsweg 23 NL-2031 NC Haarlem The Netherlands

Manufacturing plant

Usine edilon)(sedra 600 712

This European Technical Assessment contains

22 pages including 18 Annexes which form an integral part of this assessment.

This European Technical Assessment is issued in accordance with regulation (EU) No 305/2011, on the basis of

ETAG 001-Part 1 and Part 5, edition 2013, used as European Assessment Document (EAD)

Translations of this European Technical Assessment in other languages shall fully correspond to the original issued document and should be identified as such.

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# 1. Technical description of the product

The edilon)(sedra Dex<sup>®</sup>-EA 2K, edilon)(sedra Dex<sup>®</sup>-EA 2KC (faster curing time) and edilon)(sedra Dex<sup>®</sup>-EA 2KH (extended processing time) with steel elements is bonded anchor (injection type).

Steel elements can be galvanized or stainless steel threaded rod or rebar.

Steel element is placed into a drilled hole filled with injection mortar. The steel element is anchored via the bond between metal part, injection mortar and concrete. The anchor is intended to be used with embedment depth from 8 diameters to 20 diameters.

The illustration and the description of the product are given in Annex A.

# 2. Specification of the intended use in accordance with the applicable EAD

The performances given in Section 3 are only valid if the anchor is used in compliance with the specifications and conditions given in Annex B.

The provisions made in this European Technical Assessment are based on an assumed working life of the anchor of 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the products in relation to the expected economically reasonable working life of the works.

# 3. Performance of the product and references to the methods used for its assessment

3.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic resistance for tension loads - threaded rod	See Annex C 1
Characteristic resistance for tension loads - rebar	See Annex C 2
Characteristic resistance for shear loads - threaded rod	See Annex C 3
Characteristic resistance for shear loads - rebar	See Annex C 4
Characteristic resistance for tension loads - threaded rod	See Annex C 5
Characteristic resistance for tension loads - rebar	See Annex C 6
Characteristic resistance for shear loads - threaded rod	See Annex C 7
Characteristic resistance for shear loads - rebar	See Annex C 8
Displacement for threaded rod	See Annex C 9
Displacement for rebar	See Annex C 10

3.2 Safety in case of fire (BWR 2)

Essential characteristic	Performance
Reaction to fire	Anchorages satisfy
	requirements for
	Class A1
Resistance to fire	No performance determined

# 3.3 Hygiene, health and environment (BWR 3)

Regarding dangerous substances contained in this European Technical Assessment, there may be requirements applicable to the products falling within its scope (e.g. transposed European legislation and national laws, regulations and administrative provisions). In order to meet the provisions of the Regulation (EU) No 305/2011, these requirements need also to be complied with, when and where they apply.

## 3.4 Safety in use (BWR 4)

For basic requirement safety in use the same criteria are valid as for Basic Requirement Mechanical resistance and stability.

# 3.5 Protection against noise (BWR 5)

Not relevant.

# 3.6 Energy economy and heat retention (BWR 6)

Not relevant.

# 3.7 Sustainable use of natural resources (BWR 7)

For the sustainable use of natural resources no performance was determined for this product.

# 3.8 General aspects relating to fitness for use

Durability and serviceability are only ensured if the specifications of intended use according to Annex B 1 are kept.

# 4. Assessment and verification of constancy of performance (AVCP) system applied with reference to its legal base

According to the Decision 96/582/EC of the European Commission<sup>1</sup> the system of assessment verification of constancy of performance (see Annex V to Regulation (EU) No 305/2011) given in the following table apply.

Product	Intended use	Level or class	System
Metal anchors for use	For fixing and/or supporting to		
in concrete	concrete, structural elements		1
	(which contributes to the stability	-	'
	of the works) or heavy units		

# 5. Technical details necessary for the implementation of the AVCP system, as provided in the applicable EAD

#### 5.1 Tasks of the manufacturer

The manufacturer shall exercise permanent internal control of production. All the elements, requirements and provisions adopted by the manufacturer shall be documented in a systematic manner in the form of written policies and procedures, including records of results performed. This production control system shall insure that the product is in conformity with this European Technical Assessment.

The manufacturer may only use raw materials stated in the technical documentation of this European Technical Assessment.

The factory production control shall be in accordance with the control plan which is a part of the technical documentation of this European Technical Assessment. The control plan is laid down in the context of the factory production control system operated by the manufacturer and deposited at Technický a zkušební ústav stavební Praha, s.p.<sup>2</sup> The results of factory production control shall be recorded and evaluated in accordance with the provisions of the control plan.

The manufacturer shall, on the basis of a contract, involve a body which is notified for the tasks referred to in section 4 in the field of anchors in order to undertake the actions laid down in section 5.2. For this purpose, the control plan referred to in this section and section 5.2 shall be handed over by the manufacturer to the notified body involved.

The manufacturer shall make a declaration of performance, stating that the construction product is in conformity with the provisions of this European Technical Assessment.

1

Official Journal of the European Communities L 254 of 08.10.1996

The control plan is a confidential part of the documentation of the European Technical Assessment, but not published together with the ETA and only handed over to the approved body involved in the procedure of AVCP.

### 5.2 Tasks of the notified bodies

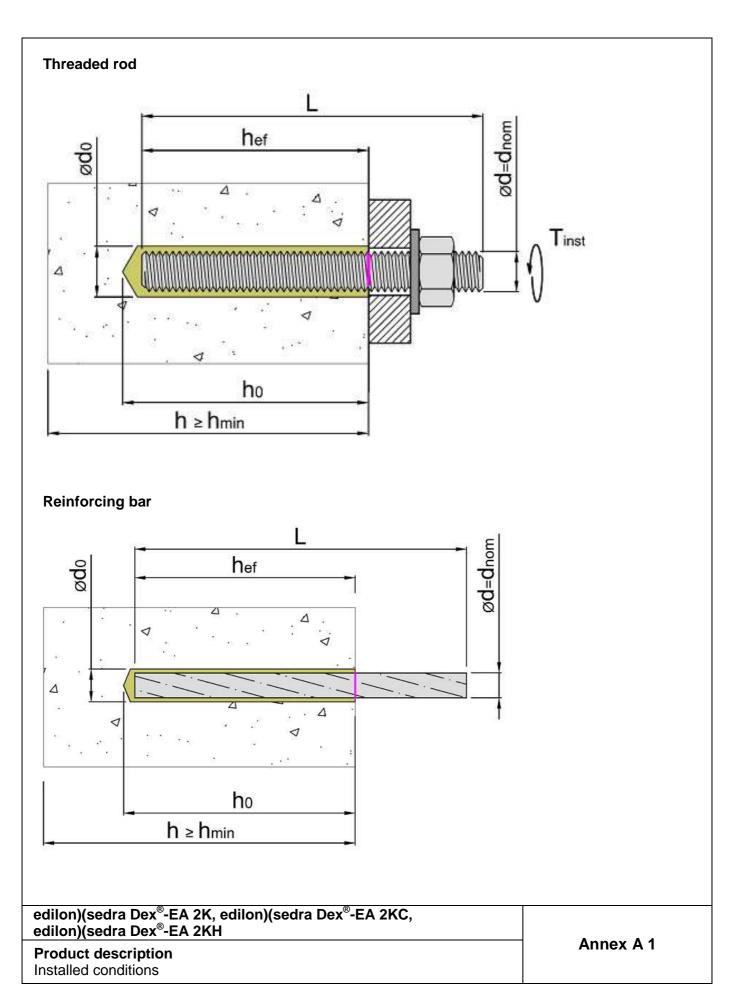
The notified body shall retain the essential points of its actions referred to above and state the results obtained and conclusions drawn in a written report.

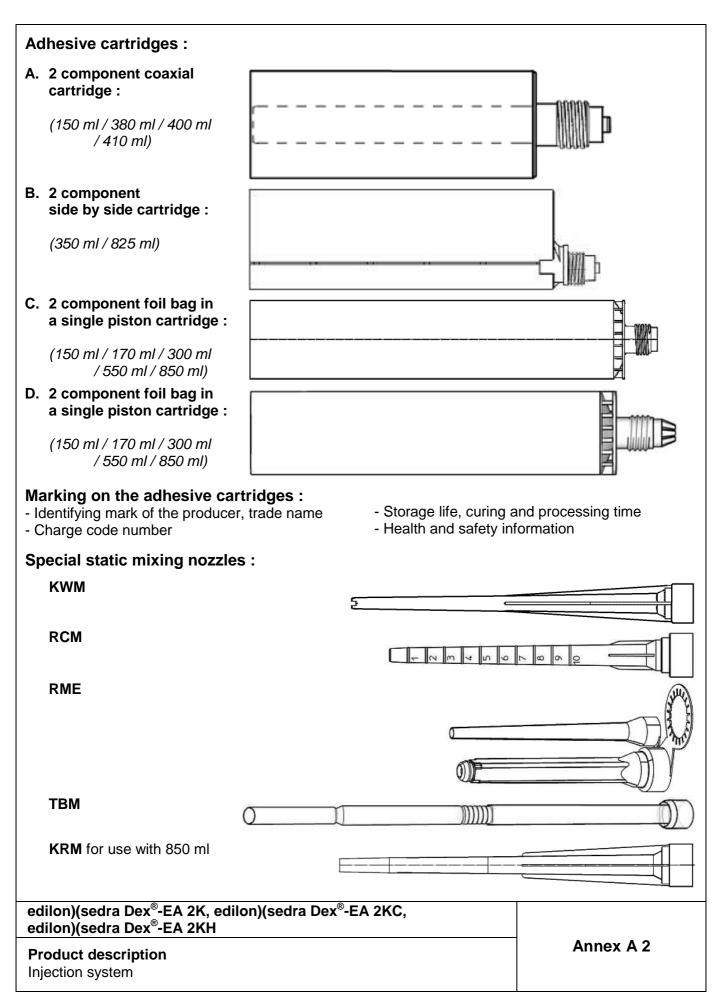
The notified certification body involved by the manufacturer shall issue an certificate of constancy of performance of the product stating the conformity with the provisions of this European Technical Assessment.

In cases where the provisions of the European Technical Assessment and its control plan are no longer fulfilled the notified body shall withdraw the certificate of constancy of performance and inform Technický a zkušební ústav stavební Praha, s.p without delay.

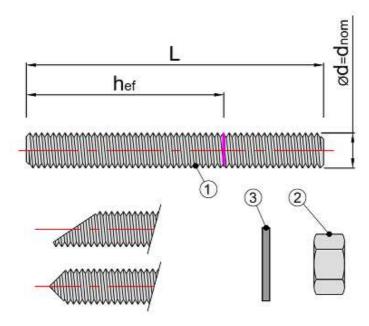
Issued in Prague on 21.08.2014

signed by
Ing. Václav Hadrava
Head of the department Technical Assessment Body





# Threaded rod M8, M10, M12, M16, M20, M24, M27, M30



Standard commercial threaded rod with marked embedment depth

Carbon Steel, Zinc electroplated coating ≥ 5 μm according EN-ISO 4042 Carbon Steel, Hot dip galvanized coating ≥ 40 μm according EN-ISO 1461 and EN-ISO10684							
Part	Part Designation Material and EN / ISO reference						
1.	1. Threaded rod according to EN 10087 or EN 10263 grade 4.6, 5.8, 8.8 and 10.9 acc. to EN-ISO 898-1						
2.	Hexagon nut M8 to M30	EN-ISO 4032, Steel according to threaded rod grade according to EN-ISO 898-2					
3.	Washer	EN ISO 887, EN-ISO 7089, EN-ISO 7093 or EN-ISO 7094, Steel according to threaded rod					

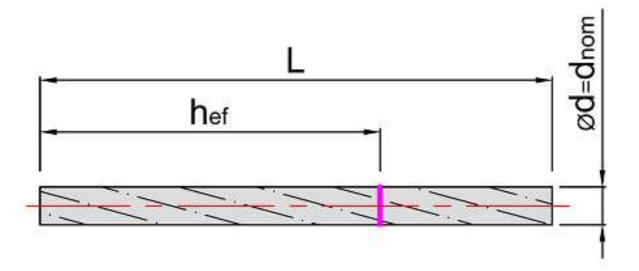
<sup>\*</sup>Galvanized rod of high strength are sensitive to hydrogen induced brittle failure

Stainless Steel					
Part	Designation	Material and EN / ISO reference			
1.	Threaded rod M8 to M30	<b>Stainless Steel,</b> 1.4401, 1.4404 or 1.4571 according to EN 10088 grade A4-70 or A4-80, according to EN-ISO 3506-1			
2.	Hexagon nut M8 to M30	EN-ISO 4032, Stainless steel according to threaded rod grade A4-70, A4-80, according to EN-ISO 3506-2			
3.	Washer	EN ISO 887, EN-ISO 7089, EN-ISO 7093 or EN-ISO 7094, Stainless steel according to threaded rod			

High o	High corrosion resistant Stainless Steel 1.4529					
Part	art Designation Material and EN / ISO reference					
1.	Threaded rod M8 to M30	High corrosion resistant Stainless Steel, 1.4529 according to EN 10088 grade A4-70 or A4-80, according to EN-ISO 3506-1				
2.	Hexagon nut M8 to M30	EN-ISO 4032, Stainless steel according to threaded rod grade A4-70, A4-80, according to EN-ISO 3506-2				
3.	Washer	EN ISO 887, EN-ISO 7089, EN-ISO 7093 or EN-ISO 7094, Stainless steel according to threaded rod				

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Product description Threaded rod and materials	Annex A 3

Rebar Ø8, Ø10, Ø12, Ø16, Ø20, Ø25, Ø32



Standard commercial reinforcing bar with marked embedment depth

Product form	Bars and de-coiled rods				
Class	В	С			
Characteristic yield strength fyk or fo	400 to 600				
Minimum value of $k = (f_t / f_y)_k$	≥ 1,08 ≥ 1,15 < 1,35				
Characteristic strain at maximum fo	≥ 5,0 ≥ 7,5				
Bendability	Bendability				
Maximum deviation from nominal mass (individual bar or wire) (%)	Nominal bar size (mm) ≤ 8	± 6,0			
	± 4	1,5			
Bond: Minimum relative rib area, $f_{R,min}$	Nominal bar size (mm) 8 to 12	0,040			
	> 12	0,0	56		

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Product description Rebars and materials	Annex A 4

# Specifications of intended use

# **Anchorages subject to:**

Static and quasi-static load.

#### **Base materials**

- Non-cracked concrete.
- Cracked and non-cracked concrete for threaded rod size M10, M12, M16, M20, M24
- Reinforced or unreinforced normal weight concrete of strength class C20/25 at minimum and C50/60 at maximum according EN 206-1:2000-12.

# Temperature range:

• -40°C to +80°C (max. short. term temperature +80°C and max. long term temperature +50°C)

# **Use conditions (Environmental conditions)**

- Structures subject to dry internal conditions (zinc coated steel, stainless steel, high corrosion resistance steel).
- Structures subject to external atmospheric exposure including industrial and marine environment, if no particular aggressive conditions exist (stainless steel, high corrosion resistance steel).
- Structures subject to permanently damp internal condition, if no particular aggressive conditions exist (stainless steel, high corrosion resistance steel).
- Structures subject to permanently damp internal condition, with particular aggressive conditions exist (high corrosion resistance steel).

Note: Particular aggressive conditions are e.g. permanent, alternating immersion in seawater or the splash zone of seawater, chloride atmosphere of indoor swimming pools or atmosphere with extreme chemical pollution (e.g. in desulphurization plants or road tunnels where de-icing materials are used).

### **Use categories:**

• Category 2 – installation in dry or wet concrete or in flooded hole.

#### Design:

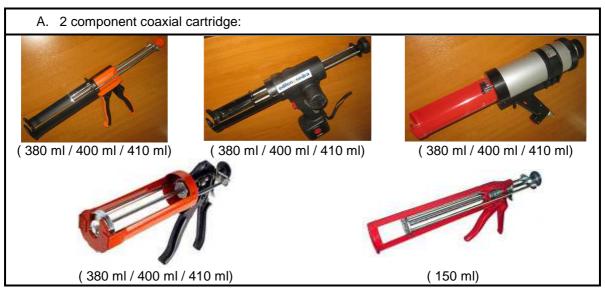
- The anchorages are designed in accordance with the EOTA Technical Report TR 029 "Design of bonded anchors" under the responsibility of an engineer experienced in anchorages and concrete work
- Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored. The position of the anchor is indicated on the design drawings.

# Installation:

- Dry or wet concrete or flooded hole.
- Hole drilling by rotary drill mode.
- Anchor installation carried out by appropriately qualified personnel and under the supervision of the person responsible for technical matters of the site.

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Intended use Specifications	Annex B 1

# **Dispensing applicators:**







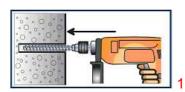
# Steel-polymer cleaning brush:



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Intended use	Annex B 2
Applicator guns	
Cleaning brush	

#### Installation instructions

Drill the hole to the correct diameter and depth using a rotary percussion drilling machine.

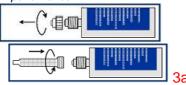


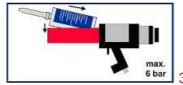
Thoroughly clean the hole in the 2 following sequence using a steelpolymer cleaning brush with the required extensions and a hand blow pump:

Blow Clean x2. Brush Clean x2.

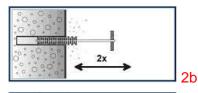
Blow Clean x2. Brush Clean x2. Blow Clean x2.

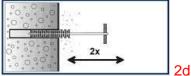
Remove standing water from the hole prior to cleaning to achieve maximum performance

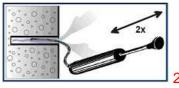




2c





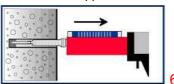


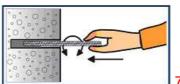
CHECK

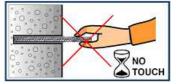
3 Select the appropriate static mixing nozzle for the installation, open the cartridge/cut foil pack and screw nozzle onto the mouth of the cartridge. Insert the cartridge into a good quality dispensing applicator (gun).

Extrude the first part of the cartridge to waste until an even colour has been achieved without streaking in the resin. 4

If necessary, cut the extension tube to the depth of the hole and push onto the end of the mixing nozzle, and fit the 5 correct resin stopper to the other end.







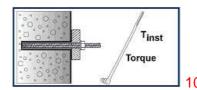
Insert the mixing nozzle (or the extension tube with resin stopper when necessary) to the bottom of the hole. Begin to extrude the resin and slowly withdraw the mixing nozzle from the hole ensuring that there are no air voids as the mixing nozzle is withdrawn. Fill the hole to approximately ½ to ¾ full and withdraw the nozzle completely.

Insert the clean threaded rod or rebar, free from oil or other release agents, to the bottom of the hole using a back 7 and forth twisting motion ensuring all the threads are thoroughly coated. Adjust to the correct position within the stated working time.

Excess resin will be expelled from the hole evenly around the steel element showing that the hole is full. 8 This excess resin should be removed from around the mouth of the hole before it sets.

9 Leave the anchor to cure. Do not disturb the anchor until the appropriate loading time has elapsed depending on the substrate conditions and ambient temperature.

Attach the fixture and tighten the nut to the recommended torque. Do not overtighten.



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Intended use

Installation procedure

Annex B 3

Table B1: Installation parameters of threaded rod

Size			M8	M10	M12	M16	M20	M24	M27	M30
Nominal drill hole diameter	$ \emptyset d_0 $	[mm]	10	12	14	18	22	26	30	35
Diameter of cleaning brush	d <sub>b</sub>	[mm]	14	14	20	20	29	29	40	40
Torque moment	$T_{inst}$	[Nm]	10	20	40	80	150	200	240	275
$h_{ef,min} = 8d$										
Depth of drill hole	$h_0$	[mm]	64	80	96	128	160	192	216	240
Minimum edge distance	C <sub>min</sub>	[mm]	35	40	50	65	80	96	110	120
Minimum spacing	Smin	[mm]	35	40	50	65	80	96	110	120
Minimum thickness of member	$h_{min}$	[mm]	h <sub>ef</sub> -	⊦ 30 mn	n ≥ 100	mm		h <sub>ef</sub> +	- 2d <sub>0</sub>	
$h_{ef,max} = 20d$										
Depth of drill hole	$h_0$	[mm]	160	200	240	320	400	480	540	600
Minimum edge distance	C <sub>min</sub>	[mm]	80	100	120	160	200	240	270	300
Minimum spacing	Smin	[mm]	80	100	120	160	200	240	270	300
Minimum thickness of member	$h_{min}$	[mm]	h <sub>ef</sub> -	⊦ 30 mn	n ≥ 100	mm	h <sub>ef</sub> + 2d <sub>0</sub>			

Table B2: Installation parameters of rebar

Table bz. Installation parameters	ui iebai								
Size			Ø8	Ø10	Ø12	Ø16	Ø20	Ø25	Ø32
Nominal drill hole diameter	$ \emptyset d_0 $	[mm]	12	14	16	20	25	32	40
Diameter of cleaning brush	$d_b$	[mm]	14	14	19	22	29	40	42
$h_{ef,min} = 8d$									
Depth of drill hole	$h_0$	[mm]	64	80	96	128	160	200	256
Minimum edge distance	C <sub>min</sub>	[mm]	35	40	50	65	80	100	130
Minimum spacing	Smin	[mm]	35	40	50	65	80	100	130
Minimum thickness of member	$h_{min}$	[mm]	h <sub>ef</sub>	+ 30 mn	n ≥ 100 ı	mm	h <sub>ef</sub> + 2d <sub>0</sub>		
$h_{ef,max} = 20d$									
Depth of drill hole	$h_0$	[mm]	160	200	240	320	400	500	640
Minimum edge distance	C <sub>min</sub>	[mm]	80	100	120	160	200	250	320
Minimum spacing	S <sub>min</sub>	[mm]	80	100	120	160	200	250	320
Minimum thickness of member	$h_{min}$	[mm]	h <sub>ef</sub>	+ 30 mn	n ≥ 100 ı	mm		h <sub>ef</sub> + 2d <sub>0</sub>	)

# Table B3: Cleaning

All diameters
- 2 x blowing
- 2 x brushing
- 2 x blowing
- 2 x brushing
- 2 x blowing

Table B4: Minimum curing time

edilon)(sedra Dex®-EA 2l	edilon)(sedra Dex <sup>®</sup> -EA 2K								
Application temperature	Processing time	Load time							
+5 to +10°C	10 mins	145 mins							
+10 to +15°C	8 mins	85 mins							
+15 to +20°C	6 mins	75 mins							
+20 to +25°C	5 mins	50 mins							
+25 to +30°C	4 mins	40 mins							

Processing time refers to the highest temperature in the range. Load time refers to the lowest temperature in the range. Cartridge must be conditioned to a minimum +5°C.

edilon)(sedra Dex <sup>®</sup> -EA 2KC							
Application temperature	Processing time	Load time					
-10 to -5°C	50 mins	12 hours					
-5 to 0°C	15 mins	100 mins					
0 to +5°C	10 mins	75 mins					
+5 to +20°C	5 mins	50 mins					
> +20°C	100 second	20 mins					

Processing time refers to the highest temperature in the range. Load time refers to the lowest temperature in the range.

edilon)(sedra Dex <sup>®</sup> -EA 2KH								
Application temperature	Processing time	Load time						
+15 to +20°C	15 mins	5 hours						
+20 to +25°C	10 mins	145 mins						
+25 to +30°C	7.5 mins	85 mins						
+30 to +35°C	5 mins	50 mins						
+35 to +40°C	3.5 mins	40 mins						

Processing time refers to the highest temperature in the range. Load time refers to the lowest temperature in the range. Cartridge must be conditioned to a minimum +15°C.

Cartridge must be				
edilon)(sedra	Dex®-EA 2K,	edilon)(sedra	Dex®-EA	2KC,

edilon)(sedra Dex®-EA 2KH

Intended use

Installation parameters

Curing time

Annex B 4

**Table C1:** Design method TR 029 Characteristic values of resistance to tension load of threaded rod

Steel failure - Characteristic res	istance									
Size			M8	M10	M12	M16	M20	M24	M27	M30
Steel grade <b>4.6</b>	$N_{Rk,s}$	[kN]	15	23	34	63	98	141	184	224
Partial safety factor	γ <sub>Ms</sub>	[-]				2	2			
Steel grade 5.8	$N_{Rk,s}$	[kN]	18	29	42	79	123	177	230	281
Partial safety factor	γ <sub>Ms</sub>	[-]				1	,5			
Steel grade 8.8	$N_{Rk,s}$	[kN]	29	46	67	126	196	282	367	449
Partial safety factor	γMs	[-]				1	,5			
Steel grade 10.9	$N_{Rk,s}$	[kN]	37	58	84	157	245	353	459	561
Partial safety factor	γ <sub>Ms</sub>	[-]				1	,4			
Stainless steel grade A4-70	$N_{Rk,s}$	[kN]	26	41	59	110	172	247	321	393
Partial safety factor	γ <sub>Ms</sub>	[-]				1	,9			
Stainless steel grade A4-80	$N_{Rk,s}$	[kN]	29	46	67	126	196	282	367	449
Partial safety factor	γ <sub>Ms</sub>	[-]	1,6							
Stainless steel grade 1.4529	$N_{Rk,s}$	[kN]	26	41	59	110	172	247	321	393
Partial safety factor	γ <sub>Ms</sub>	[-]				1	,5			

Size			М8	M10	M12	M16	M20	M24	M27	M30
Characteristic bond resistance	in non-cra	cked conc	rete							
Dry and wet concrete	$ au_{Rk}$	[N/mm <sup>2</sup> ]	10	9,5	9,5	9	8,5	8	6,5	5,5
Partial safety factor	γ <sub>Mc</sub> 1)	[-]			1,	8 <sup>2)</sup>	•		2,	1 <sup>3)</sup>
Flooded hole	$ au_{Rk}$	[N/mm <sup>2</sup> ]	8,5	7,5	7	7	6,5	5,5		
Partial safety factor	γ <sub>Mc</sub>	[-]	2,1 <sup>3)</sup>							
Factor for concrete C50/60	Ψc	[-]	1							

Combined pullout and con	crete co	ne failu	ire in crack	ed conc	rete C20/2	25		
Size				M10	M12	M16	M20	M24
Characteristic bond resista	ance in c	racked	concrete					
Dry and wet concrete		$\tau_{Rk}$	[N/mm <sup>2</sup> ]	4,5	4,5	4,5	4	4
Partial safety factor		γ <sub>Mc</sub> 1)	[-]			1,8 <sup>2)</sup>		
Flooded hole		$\tau_{Rk}$	[N/mm <sup>2</sup> ]	4,5	4,5	4,5	4	4
Partial safety factor		γ <sub>Mc</sub> 1)	[-]			2,1 <sup>3)</sup>		
	C30/37					1,12		
Factor for cracked concrete	C40/50	$\psi_{c}$	[-]			1,23		
	C50/60					1,30		

Splitting failure										
Size				M8 M10 M12 M16 M20 M24 M27 M						
Edge distance	C <sub>cr,sp</sub>	[mm]	1,5h <sub>ef</sub>							
Spacing	S <sub>cr,sp</sub>	[mm]	3,0h <sub>ef</sub>							
Partial safety factor	1) γMsp	[-]	1,8							

<sup>&</sup>lt;sup>1)</sup> In absence of national regulations <sup>2)</sup> The partial safety factor  $\gamma_2$ =1,2 is included <sup>3)</sup> The partial safety factor  $\gamma_2$ =1,4 is included

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Performances	Annex C 1
Design according to TR 029	
Characteristic resistance for tension loads - threaded rod	

Table C2: Design method TR 029 Characteristic values of resistance to tension load of rebar

Steel failure - Characteristic resi	stance								
Size			Ø8	Ø10	Ø12	Ø16	Ø20	Ø25	Ø32
Rebar BSt 500 S	$N_{Rk,s}$	[kN]	28	43	62	111	173	270	442
Partial safety factor	γ <sub>Ms</sub>	[-]				1,4			

Combined pullout and concrete	Combined pullout and concrete cone failure in non-cracked concrete C20/25										
Size			Ø8	Ø10	Ø12	Ø16	Ø20	Ø25	Ø32		
Characteristic bond resistance in non-cracked concrete											
Dry and wet concrete	$\tau_{Rk}$	[N/mm <sup>2</sup> ]	11	9,5	9,5	9	8,5	8,5	5,5		
Partial safety factor	γ <sub>Mc</sub> 1)	[-]				1,8 <sup>2)</sup>					
Flooded hole	$ au_{Rk}$	[N/mm <sup>2</sup> ]	11	9,5	9,5	9	8,5	8,5	5,5		
Partial safety factor	γ <sub>Mc</sub> 1)	[-]				2,1 <sup>3)</sup>					
Factor for concrete C50/60	Ψc	[-]				1					

Splitting failure									
Size	-		Ø8	Ø10	Ø12	Ø16	Ø20	Ø25	Ø32
Edge distance	C <sub>cr,sp</sub>	[mm]				1,5h <sub>ef</sub>			
Spacing	S <sub>cr,sp</sub>	[mm]	3,0h <sub>ef</sub>						
Partial safety factor	1) γ <sub>Msp</sub>	[-]				1,8			

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Performances	Annex C 2
Design according to TR 029	
Characteristic resistance for tension loads - rebar	

<sup>&</sup>lt;sup>1)</sup> In absence of national regulations
<sup>2)</sup> The partial safety factor  $\gamma_2$ =1,2 is included
<sup>3)</sup> The partial safety factor  $\gamma_2$ =1,4 is included

**Table C3:** Design method TR 029 Characteristic values of resistance to shear load of threaded rod

Steel failure without lever arm										
Size			M8	M10	M12	M16	M20	M24	M27	M30
Steel grade 4.6	$V_{Rk,s}$	[kN]	7	12	17	31	49	71	92	112
Partial safety factor	$\gamma_{Ms}^{\hspace{0.1cm}1)}$	[-]				1,	67			
Steel grade 5.8	$V_{Rk,s}$	[kN]	9	15	21	39	61	88	115	140
Partial safety factor	γ <sub>Ms</sub>	[-]				1,	25			
Steel grade 8.8	$V_{Rk,s}$	[kN]	15	23	34	63	98	141	184	224
Partial safety factor	γ <sub>Ms</sub>	[-]				1,	25			
Steel grade 10.9	$V_{Rk,s}$	[kN]	18	29	42	79	123	177	230	281
Partial safety factor	γ <sub>Ms</sub>	[-]				1	,5			
Stainless steel grade A4-70	$V_{Rk,s}$	[kN]	13	20	30	55	86	124	161	196
Partial safety factor	γ <sub>Ms</sub> 1)	[-]				1,	56			
Stainless steel grade A4-80	$V_{Rk,s}$	[kN]	15	23	34	63	98	141	184	224
Partial safety factor	γ <sub>Ms</sub> 1)	[-]	1,33							
Stainless steel grade 1.4529	$V_{Rk,s}$	[kN]	13	20	30	55	86	124	161	196
Partial safety factor	γ <sub>Ms</sub> 1)	[-]				1,	25			

Steel failure with lever arm										
Size			M8	M10	M12	M16	M20	M24	M27	M30
Steel grade <b>4.6</b>	$M^{o}_{Rk,s}$	[N.m]	15	30	52	133	260	449	666	900
Partial safety factor	γ <sub>Ms</sub> 1)	[-]				1,	67			
Steel grade 5.8	$M^{o}_{Rk,s}$	[N.m]	19	37	66	166	325	561	832	1125
Partial safety factor	γ <sub>Ms</sub>	[-]				1,:	25			
Steel grade 8.8	$M^{o}_{Rk,s}$	[N.m]	30	60	105	266	519	898	1332	1799
Partial safety factor	$\gamma_{Ms}^{\qquad 1)}$	[-]				1,	25			
Steel grade 10.9	$M^{o}_{Rk,s}$	[N.m]	37	75	131	333	649	1123	1664	2249
Partial safety factor	$\gamma_{Ms}^{\hspace{0.1cm}1)}$	[-]				1,	50			
Stainless steel grade A4-70	$M^{o}_{Rk,s}$	[N.m]	26	52	92	233	454	786	1165	1574
Partial safety factor	γ <sub>Ms</sub> 1)	[-]				1,	56			
Stainless steel grade A4-80	$M^{o}_{Rk,s}$	[N.m]	30	60	105	266	519	898	1332	1799
Partial safety factor	γ <sub>Ms</sub>	[-]				1,	33			
Stainless steel grade 1.4529	$M^{o}_{Rk,s}$	[N.m]	26	52	92	233	454	786	1165	1574
Partial safety factor	γ <sub>Ms</sub>	[-]				1,	25			
Concrete pryout failure										
Factor k from TR 029							2			
Design of bonded anchors, Part 5.2.3.3										
Partial safety factor	γ <sub>Mp</sub> 1)	[-]				1	,5			

Concrete edge failure											
Size	M8	M10	M12	M16	M20	M24	M27	M30			
See section 5.2.3.4 of Technical Report TR 029 for the Design of Bonded Anchors											
Partial safety factor $\gamma_{Mc}^{(1)}$ [-]				1,	,5						

<sup>1)</sup> In absence of national regulations

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Performances	Annex C 3
Design according to TR 029	
Characteristic resistance for shear loads - threaded rod	

**Table C4:** Design method TR 029 Characteristic values of resistance to shear load of rebar

Steel failure without lever arm									
Size			Ø8	Ø10	Ø12	Ø16	Ø20	Ø25	Ø32
Rebar BSt 500 S	$V_{Rk,s}$	[kN]	14	22	31	55	86	135	221
Partial safety factor	γ <sub>Ms</sub>	[-]				1,5			

Steel failure with lever arm									
Size			Ø8	Ø10	Ø12	Ø16	Ø20	Ø25	Ø32
Rebar BSt 500 S	$M^{o}_{Rk,s}$	[N.m]	33	65	112	265	518	1013	2122
Partial safety factor	γ <sub>Ms</sub>	[-]				1,5			
Concrete pryout failure	3	-							
Factor k from TR 029						2			
Design of bonded anchors, Part 5.2.3.	3								
Partial safety factor	1) γ <sub>Μp</sub>	[-]				1,5			

Concrete edge failure									
Size	Ø8	Ø10	Ø12	Ø16	Ø20	Ø25	Ø32		
See section 5.2.3.4 of Technical Report TR 029 for the Design of Bonded Anchors									
Partial safety factor $\gamma_{Mc}^{(1)}$ [-]				1,5					

<sup>1)</sup> In absence of national regulations

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Performances	Annex C 4
Design according to TR 029	
Characteristic resistance for shear loads - rebar	

**Table C5:** Design method CEN/TS 1992-4 Characteristic values of resistance to tension load of threaded rod

Steel failure - Characteristic res	istance									
Size			M8	M10	M12	M16	M20	M24	M27	M30
Steel grade <b>4.6</b>	$N_{Rk,s}$	[kN]	15	23	34	63	98	141	184	224
Partial safety factor	γMs	[-]				2	2			
Steel grade 5.8	$N_{Rk,s}$	[kN]	18	29	42	79	123	177	230	281
Partial safety factor	γ <sub>Ms</sub>	[-]				1,	,5			
Steel grade 8.8	$N_{Rk,s}$	[kN]	29	46	67	126	196	282	367	449
Partial safety factor	γ <sub>Ms</sub>	[-]				1,	,5			
Steel grade 10.9	$N_{Rk,s}$	[kN]	37	58	84	157	245	353	459	561
Partial safety factor	γMs	[-]				1,	,4			
Stainless steel grade A4-70	$N_{Rk,s}$	[kN]	26	41	59	110	172	247	321	393
Partial safety factor	γ <sub>Ms</sub>	[-]				1,	,9			
Stainless steel grade A4-80	$N_{Rk,s}$	[kN]	29	46	67	126	196	282	367	449
Partial safety factor	γMs	[-]	1,6							
Stainless steel grade 1.4529	$N_{Rk,s}$	[kN]	26	41	59	110	172	247	321	393
Partial safety factor	γMs	[-]			•	1	,5			

Combined pullout and concrete cone failure in non-cracked concrete C20/25										
Size			M8	M10	M12	M16	M20	M24	M27	M30
Characteristic bond resistance in non-cracked concrete										
Dry and wet concrete	$\tau_{Rk}$	[N/mm <sup>2</sup> ]	10	9,5	9,5	9	8,5	8	6,5	5,5
Partial safety factor	γ <sub>Mc</sub>	[-]			1,	$8^{2)}$			2,	1 <sup>3)</sup>
Flooded hole	$ au_{Rk}$	[N/mm <sup>2</sup> ]	8,5	7,5	7	7	6,5	5,5		
Partial safety factor	γ <sub>Mc</sub>	[-]				2,	1 <sup>3)</sup>			
Factor for concrete C50/60	Ψ <sub>c</sub>	[-]				1				
Factor according to CEN/TS 1992-4-5 Section 6.2.2 k <sub>8</sub>						10	,1			

Combined pullout and con	crete cor	ıre in crack	ced conc	rete C20/2	25			
Size				M10	M12	M16	M20	M24
Characteristic bond resista	ance in cı	acked	concrete					
Dry and wet concrete		$\tau_{Rk}$	[N/mm <sup>2</sup> ]	4,5	4,5	4,5	4	4
Partial safety factor		γ <sub>Mc</sub> 1)	[-]			1,8 <sup>2)</sup>		
Flooded hole		$\tau_{Rk}$	[N/mm <sup>2</sup> ]	4,5	4,5	4,5	4	4
Partial safety factor		γ <sub>Mc</sub> 1)	[-]			2,1 <sup>3)</sup>		
	C30/37					1,12		
Factor for cracked concrete	C40/50	$\Psi_{c}$	[-]			1,23		
	C50/60					1,30		
Factor according to CEN/TS 1	6.2.2 k <sub>8</sub>			7,2				

Concrete cone failure										
Size			M8	M10	M12	M16	M20	M24	M27	M30
Factor according to CEN/TS 1992-4-5	Section 6	$\frac{k_{ucr}}{k_{cr}}$		1		10 7	),1 ,2			
Edge distance	$C_{cr,N}$	[mm]				1,5	ih <sub>ef</sub>			
Spacing	$S_{cr,N}$	[mm]				3,0	h <sub>ef</sub>			
Splitting failure	-	-								
Edge distance	C <sub>cr,sp</sub>	[mm]				1,5	ih <sub>ef</sub>			
Spacing	S <sub>cr,sp.</sub>	[mm]				3,0	h <sub>ef</sub>			
Partial safety factor	γ <sub>Msp</sub>	[-]				1	8			

<sup>&</sup>lt;sup>1)</sup> In absence of national regulations <sup>2)</sup> The partial safety factor  $\gamma_2$ =1,2 is included <sup>3)</sup> The partial safety factor  $\gamma_2$ =1,4 is included

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Performances	Annex C 5
Design according to CEN/TS 1992-4	
Characteristic resistance for tension loads - threaded rod	

Table C6: Design method CEN/TS 1992-4 Characteristic values of resistance to tension load of rebar

Steel failure – Characteristic resistance										
Size			Ø8	Ø10	Ø12	Ø16	Ø20	Ø25	Ø32	
Rebar BSt 500 S	$N_{Rk,s}$	[kN]	28	43	62	111	173	270	442	
Partial safety factor	γ <sub>Ms</sub> 1)	[-]				1,4				

Combined pullout and concrete c	re in non-	cracke	d conc	rete C	20/25				
Size			Ø8	Ø10	Ø12	Ø16	Ø20	Ø25	Ø32
Characteristic bond resistance in	rete								
Dry and wet concrete	$ au_{Rk}$	[N/mm <sup>2</sup> ]	11	9,5	9,5	9	8,5	8,5	5,5
Partial safety factor	γ <sub>Mc</sub> 1)	[-]				1,8 <sup>2)</sup>			
Flooded hole	$ au_{Rk}$	[N/mm <sup>2</sup> ]	11	9,5	9,5	9	8,5	8,5	5,5
Partial safety factor	1) γ <sub>Mc</sub>	[-]				$2,1^{3)}$			
Factor for concrete C50/60	Ψс	[-]				1			
Factor according to CEN/TS 1992-4-5 Section 6.2.2 $k_8$						10,1			

Concrete cone failure									
Size			Ø8	Ø10	Ø12	Ø16	Ø20	Ø25	Ø32
Factor according to CEN/TS 1992	2-4-5 Section 6	5.2.3 k <sub>ucr</sub>				10,1			
Edge distance	$C_{cr,N}$	[mm]				1,5h <sub>ef</sub>			
Spacing	S <sub>cr,N</sub>	[mm]	3,0h <sub>ef</sub>						
Splitting failure									
Edge distance	$C_{cr,sp}$	[mm]				1,5h <sub>ef</sub>			
Spacing	S <sub>cr,sp</sub>	[mm]				3,0h <sub>ef</sub>			
Partial safety factor	γ Msp	[-]	1,8						

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Performances	Annex C 6
Design according to CEN/TS 1992-4	
Characteristic resistance for tension loads - rebar	

<sup>&</sup>lt;sup>1)</sup> In absence of national regulations <sup>2)</sup> The partial safety factor  $\gamma_2$ =1,2 is included <sup>3)</sup> The partial safety factor  $\gamma_2$ =1,4 is included

**Table C7:** Design method CEN/TS 1992-4 Characteristic values of resistance to shear load of threaded rod

Steel failure without lever arm										
Size			M8	M10	M12	M16	M20	M24	M27	M30
Steel grade <b>4.6</b>	$V_{Rk,s}$	[kN]	7	12	17	31	49	71	92	112
Partial safety factor	γ <sub>Ms</sub>	[-]				1,	67			
Steel grade 5.8	$V_{Rk,s}$	[kN]	9	15	21	39	61	88	115	140
Partial safety factor	γ <sub>Ms</sub>	[-]				1,	25			
Steel grade 8.8	$V_{Rk,s}$	[kN]	15	23	34	63	98	141	184	224
Partial safety factor	$\gamma_{Ms}^{\hspace{0.1cm}1)}$	[-]				1,	25			
Steel grade 10.9	$V_{Rk,s}$	[kN]	18	29	42	79	123	177	230	281
Partial safety factor	γ <sub>Ms</sub>	[-]				1	,5			
Stainless steel grade A4-70	$V_{Rk,s}$	[kN]	13	20	30	55	86	124	161	196
Partial safety factor	γ <sub>Ms</sub>	[-]				1,	56			
Stainless steel grade A4-80	$V_{Rk,s}$	[kN]	15	23	34	63	98	141	184	224
Partial safety factor	γ <sub>Ms</sub>	[-]				1,	33			
Stainless steel grade 1.4529	$V_{Rk,s}$	[kN]	13	20	30	55	86	124	161	196
Partial safety factor	γ <sub>Ms</sub>	[-]				1,	25			
Ductility factor according to CEN/TS 1992-4-5 Section 6.3.2.1		k <sub>2</sub>				0	,8			

Steel failure with lever arm										
Size			M8	M10	M12	M16	M20	M24	M27	M30
Steel grade 4.6	$M^{o}_{Rk,s}$	[N.m]	15	30	52	133	260	449	666	900
Partial safety factor	$\gamma_{Ms}^{}}$	[-]				1,0	67			
Steel grade 5.8	$M^{o}_{Rk,s}$	[N.m]	19	37	66	166	325	561	832	1125
Partial safety factor	γ <sub>Ms</sub>	[-]				1,	25			
Steel grade 8.8	$M^{o}_{Rk,s}$	[N.m]	30	60	105	266	519	898	1332	1799
Partial safety factor	γ <sub>Ms</sub> 1)	[-]				1,	25			
Steel grade 10.9	$M^{o}_{Rk,s}$	[N.m]	37	75	131	333	649	1123	1664	2249
Partial safety factor	γ <sub>Ms</sub>	[-]				1,	50			
Stainless steel grade A4-70	$M^{o}_{Rk,s}$	[N.m]	26	52	92	233	454	786	1165	1574
Partial safety factor	γ <sub>Ms</sub>	[-]				1,	56			
Stainless steel grade A4-80	$M^{o}_{Rk,s}$	[N.m]	30	60	105	266	519	898	1332	1799
Partial safety factor	γ <sub>Ms</sub>	[-]				1,:	33			
Stainless steel grade 1.4529	$M^{o}_{Rk,s}$	[N.m]	26	52	92	233	454	786	1165	1574
Partial safety factor	γ <sub>Ms</sub>	[-]				1,3	25			
Concrete pryout failure	-	-8								
Factor according to CEN/TS 1992-4-5	•	k <sub>3</sub>				2	,0		•	
Section 6.3.3							,0			
Partial safety factor	γ <sub>Mp</sub> 1)	[-]				1	,5			

Concrete edge failure										
Size			M8	M10	M12	M16	M20	M24	M27	M30
See section 6.3.4 of CEN/TS 1992-4-5										
Effective length of anchor	I <sub>f</sub>	[mm]			l <sub>f</sub> =	min(h	e <sub>f</sub> ;8 d <sub>n</sub>	om)		
Outside diameter of anchor	d <sub>nom</sub>	[mm]	8	10	12	16	20	24	27	30
Partial safety factor	γ <sub>Mc</sub> 1)	[-]				1	,5			

<sup>1)</sup> In absence of national regulations

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Performances	Annex C 7
Design according to CEN/TS 1992-4	
Characteristic resistance for shear loads - threaded rod	

**Table C8:** Design method CEN/TS 1992-4 Characteristic values of resistance to shear load of rebar

Steel failure without lever arm									
Size			Ø8	Ø10	Ø12	Ø16	Ø20	Ø25	Ø32
Rebar BSt 500 S	$V_{Rk,s}$	[kN]	14	22	31	55	86	135	221
Partial safety factor	γ <sub>Ms</sub> 1)	[-]				1,5			
Ductility factor according to CEN/TS 1992-4-5 Section 6.3.2.1		$k_2$				0,8			

Steel failure with lever arm									
Size			Ø8	Ø10	Ø12	Ø16	Ø20	Ø25	Ø32
Rebar BSt 500 S	$M^{o}_{Rk,s}$	[N.m]	33	65	112	265	518	1013	2122
Partial safety factor	γ <sub>Ms</sub>	[-]				1,5			
Concrete pryout failure									
Factor according to CEN/TS 1992-4-5 Section 6.3.3		<b>k</b> <sub>3</sub>				2,0			
Partial safety factor	γ <sub>Mp</sub>	[-]				1,5			

Concrete edge failure									
Size			Ø8	Ø10	Ø12	Ø16	Ø20	Ø25	Ø32
See section 6.3.4 of CEN/TS 1992-4-5									
Effective length of anchor	l <sub>f</sub>	[mm]			$I_f = m$	in(h <sub>ef</sub> ;8	d <sub>nom</sub> )		
Outside diameter of anchor	$d_{nom}$	[mm]	8	10	12	16	20	24	30
Partial safety factor	γ <sub>Mc</sub> 1)	[-]				1,5			

<sup>1)</sup> In absence of national regulations

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Performances	Annex C 8
Design according to CEN/TS 1992-4	
Characteristic resistance for shear loads - rebar	

Table C9: Displacement of threaded rod under tension and shear load

Anchor size			M8	M10	M12	M16	M20	M24	M27	M30
Non-cracked concrete										
Tension load	F	[kN]	6,3	7,9	11,9	15,9	23,8	29,8	37,7	45,6
Displacement	$\delta_{\text{N0}}$	[mm]	0,3	0,3	0,3	0,3	0,4	0,5	0,5	0,5
	$\delta_{N\infty}$	[mm]	0,5	0,5	0,5	0,5	0,5	0,5	0,5	0,5
Shear load	F	[kN]	3,1	5,0	7,2	13,5	21,0	30,3	39,4	48,0
Displacement	$\delta_{\text{V0}}$	[mm]	1,5	1,5	1,5	1,5	2,0	2,5	2,5	2,5
	$\delta_{V^\infty}$	[mm]	2,3	2,3	2,3	2,3	3,0	3,8	3,8	3,8
Cracked concrete										
Tension load	F	[kN]		5,1	7,4	13,1	20,5	24,6		
Displacement	$\delta_{\text{N0}}$	[mm]		0,4	0,7	0,7	0,7	0,6		

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Performances Displacement for threaded rod	Annex C 9

Table C10: Displacement of rebar under tension and shear load

Rebar size			Ø8	Ø10	Ø12	Ø16	Ø20	Ø25	Ø32
Non-cracked concrete									
Tension load	F	[kN]	7,9	9,9	13,9	23,8	29,8	55,6	55,6
Displacement	$\delta_{\text{N0}}$	[mm]	0,3	0,3	0,3	0,4	0,4	0,5	0,5
	$\delta_{N\infty}$	[mm]	0,5	0,5	0,5	0,5	0,5	0,5	0,5
Shear load	F	[kN]	5,9	9,3	13,3	23,7	37,0	57,9	94,8
Displacement	$\delta_{V0}$	[mm]	0,3	0,4	0,4	0,4	0,4	0,5	0,9
	$\delta_{V^{\infty}}$	[mm]	0,5	0,6	0,6	0,6	0,6	0,8	1,4

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Performances Displacement for rebar	Annex C 10